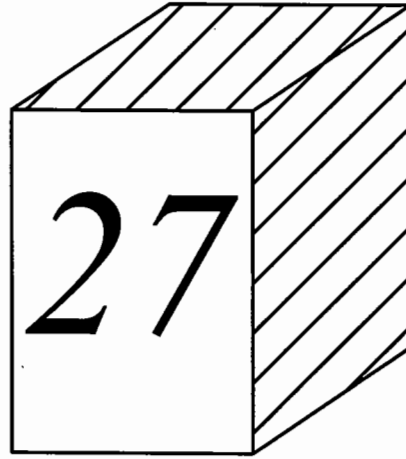


Steel رابہ
نسبہ

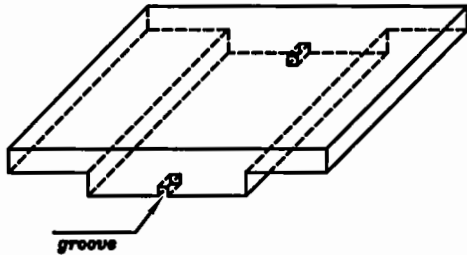
1,0.

27

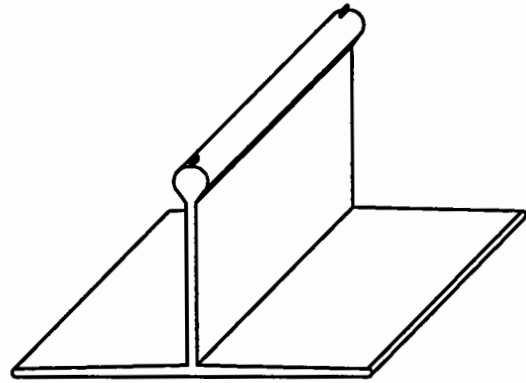


Hinged Bearing

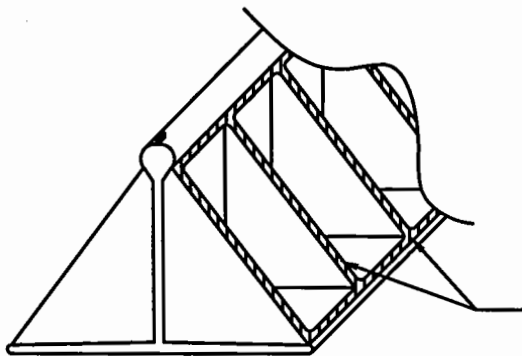
Bridge Bearing (Hinged Type)



(1) Sole Plate



(2) In the shape of the rail stiffened with ribs .



(3) Ribs for stiffen of the web.

1-Sole Plate:

يتم عمل تصميم له مثل ال Roller تماما

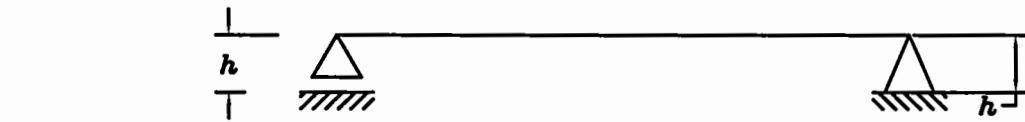
Assume L (40 to 50 cm)

$$b = b_1 + 8 \text{ cm.}$$

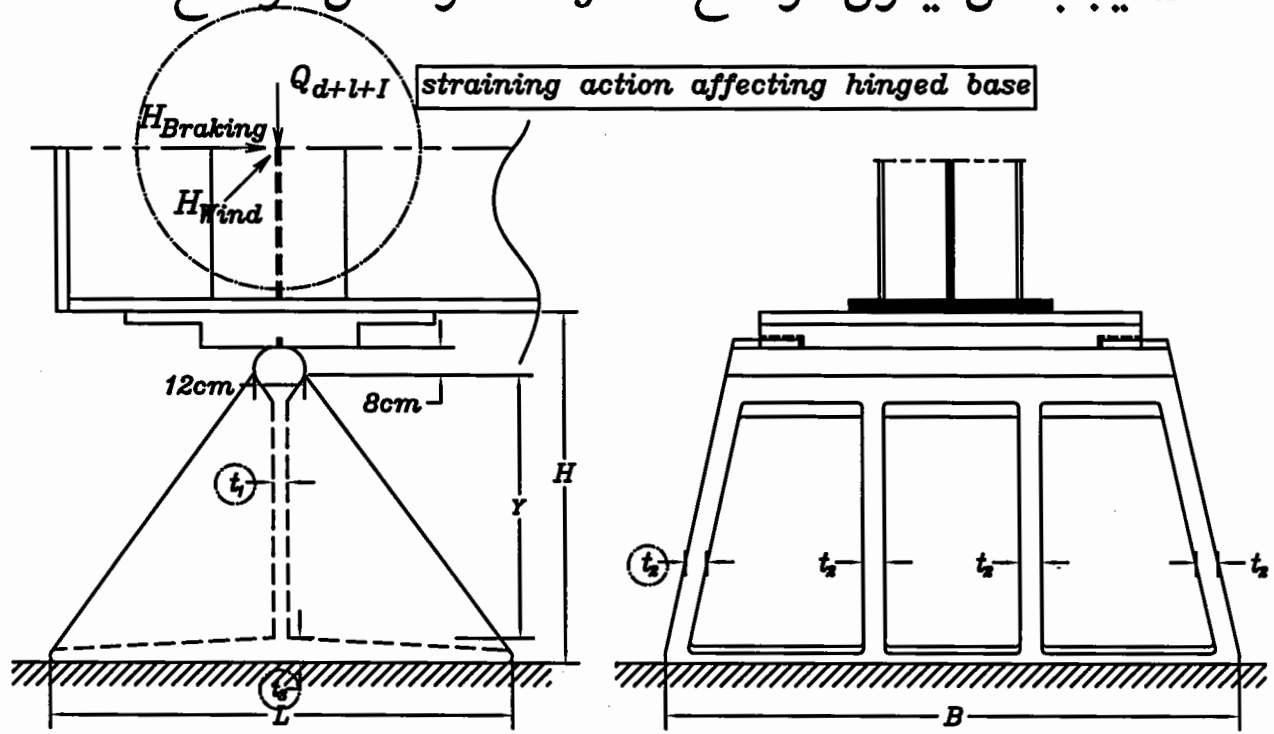
*Calculate "e" \longrightarrow $M = R/2 * e$

$$\text{Get } t_{\text{sole plate}} = \sqrt{\frac{6M}{b_1 * 1.8}}$$

2-Block:



لاحظ انه يجب ان يكون ارتفاع ال Hinge هو نفس ارتفاع ال Roller



في الامتحان اذا كانت H غير معطاة يتم فرضها بـ 40cm .

$$H_{\text{hinged}} = H_{\text{Roller}} = (t_1 + t_2 + t_3 + d)$$

Sole Plate

Upper Bearing Plate

Lower Bearing Plate

Roller

(t_1) → Thickness of the Stem.

(t_2) → Thickness of the Rib.

(t_3) → Thickness of the Base.

plates thickness for hinged base

Approximate dimensions are given below:

$$\Sigma t_2 = B/5$$

$$t_1 = H/6-8$$

$$t_3 = H/5$$

Assume dimension of the block $L \times B$

$$L \cong B \cong 1.2b_{\text{roller}}$$

($b_{\text{roller}} = b_{\text{flange}} + 20\text{cm.}$) \longrightarrow if b_{roller} not given or not calculated before

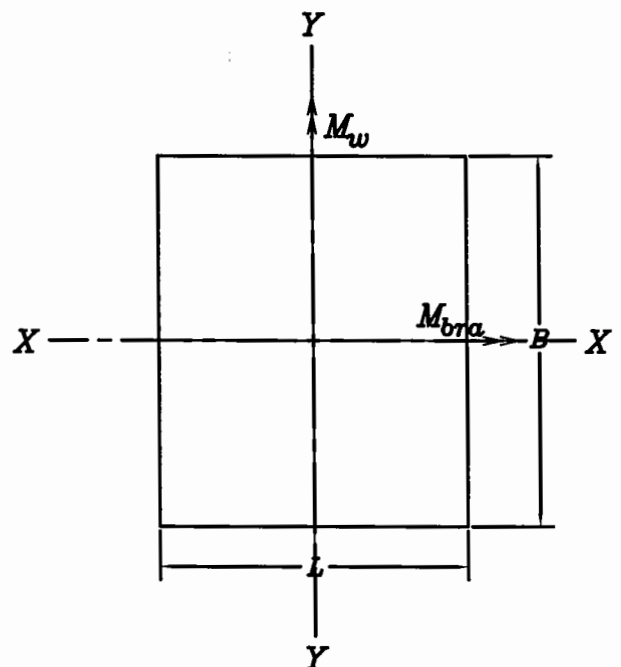
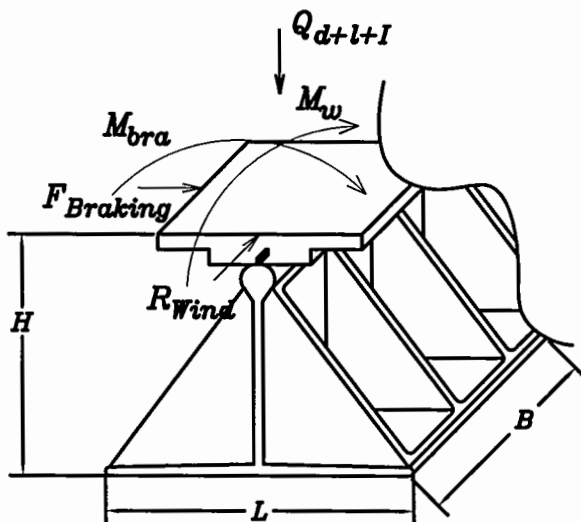
تم عمل فرض لابعاد القاعدة و على هذا يتم عمل check على ان الابعاد سليمة .

-Design of Hinged Block:

-The Hinged Block is subjected to:

(1) $R = \text{Vertical Reaction of Main Girder} = Q_{d+l+I}$

(2) $M_{\text{Wind}} = R_{\text{Wind}} \times H$, $M_{\text{Braking}} = F_{\text{Braking}} \times H$



-Calculation of R_{wind} : (moment of wind about X-X)

a-For deck bridge with 2 bracings: (X-frame)

$$R_{wind} = R_u/2 + R_l/2$$

$$\text{Where } R_u = W_{u_{loaded}} * L/2, R_l = W_{l_{loaded}} * L/2$$

b-For deck bridge with one bracing: (inverted U-frame)

$$R_{wind} = R/2$$

$$\text{Where } R = W_{loaded} * L/2$$

c-For pony bridge with one bracing:

$$R_{wind} = R/2$$

$$\text{Where } R = W_{loaded} * L/2$$

-Calculation of $R_{braking}$: (moment of braking about Y-Y)

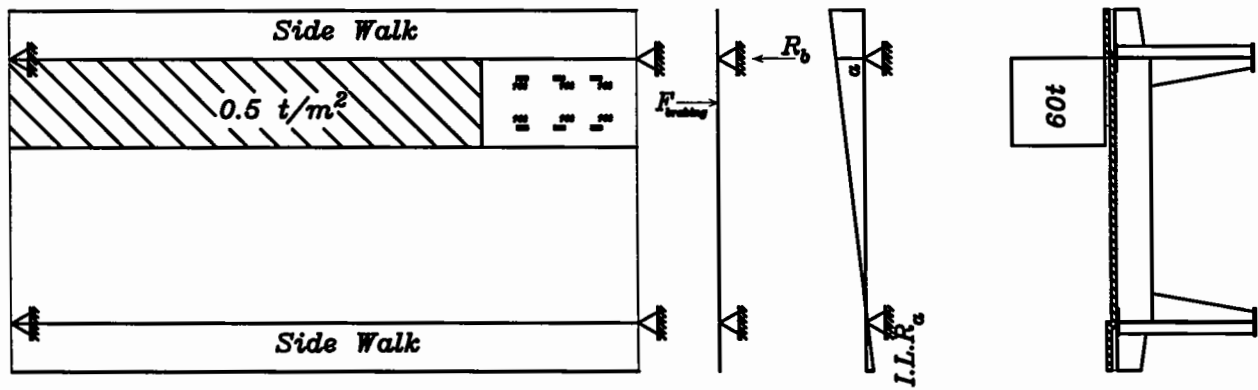
Road Way

$$F_{braking} = 1/4 (\text{Vertical Loads of Main Lane without Impact}) < 90t$$

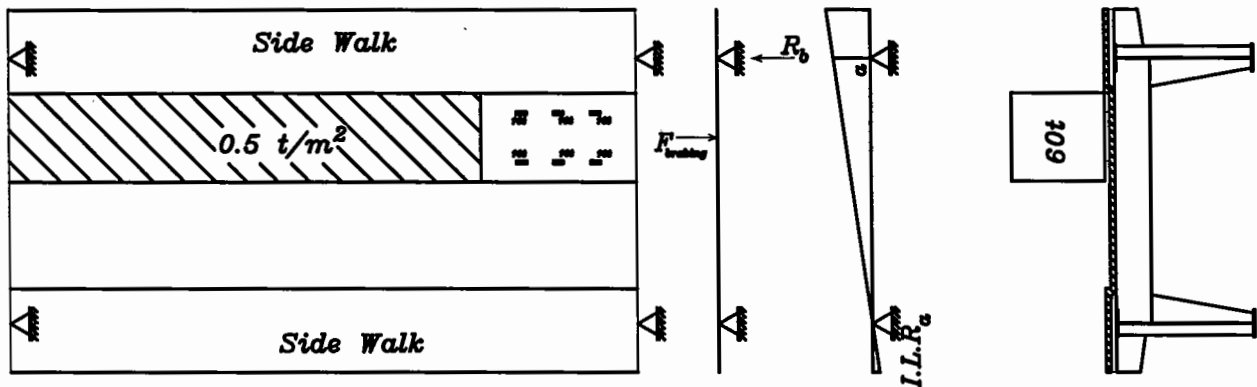
$$\text{Vertical Loads of Main Lane for Road Way} = 60 + \{(L-6) * 3 * 0.5\}$$

$R_{braking}$ = Distribution of braking on the 2 supports as a simple beam taking into consideration the critical case of loading and assume the braking force at the middle of the lane.

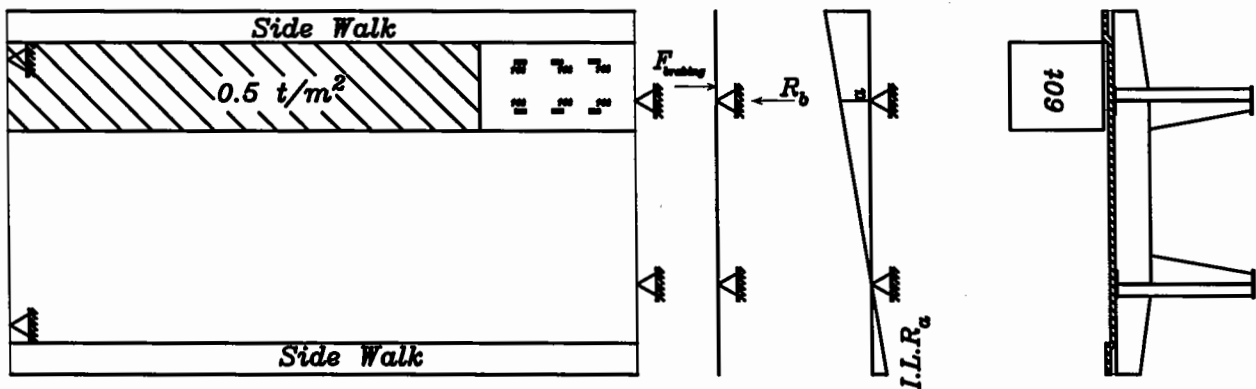
-Calculation of $R_{breaking}$ case of road way bridge



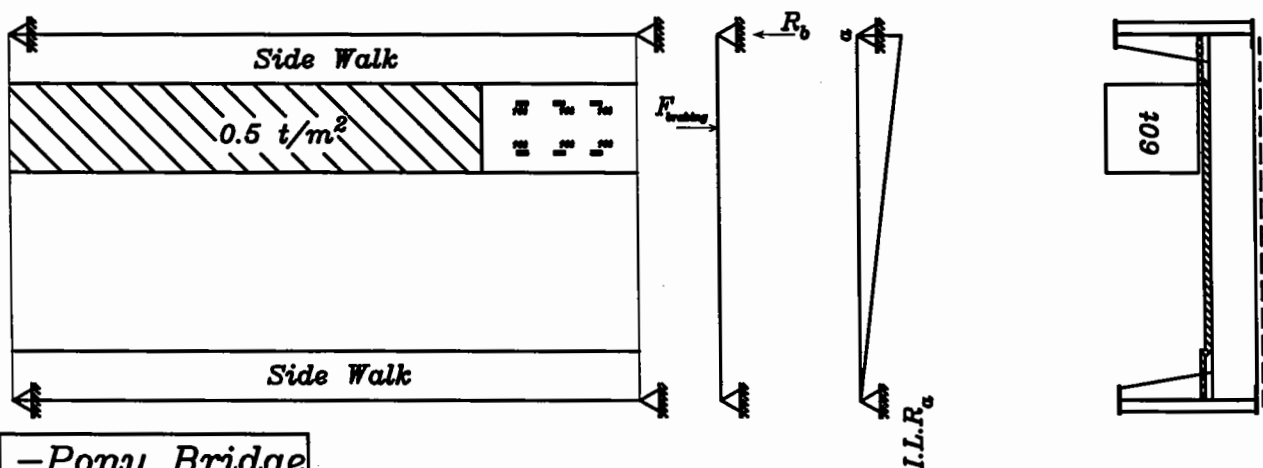
-Deck Bridge with Side Walk=Cant.



-Deck Bridge with Side Walk < Cant.ilever



-Deck Bridge with Side Walk < Cant.ilever

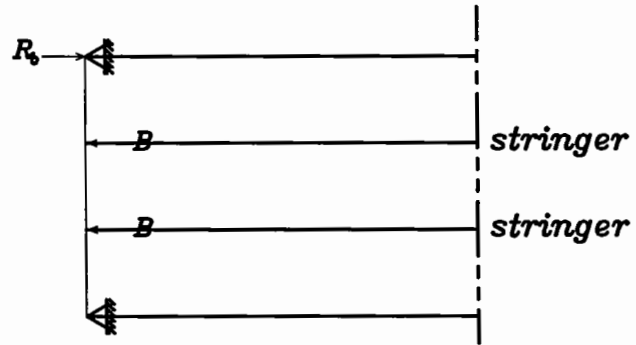


-Pony Bridge

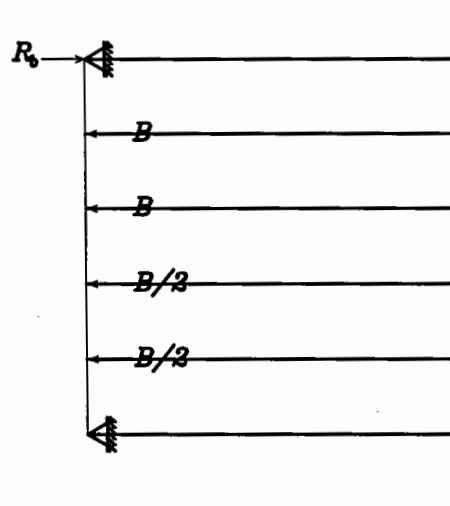
-Calculation of $R_{breaking}$ case of rail way bridge

Single track:

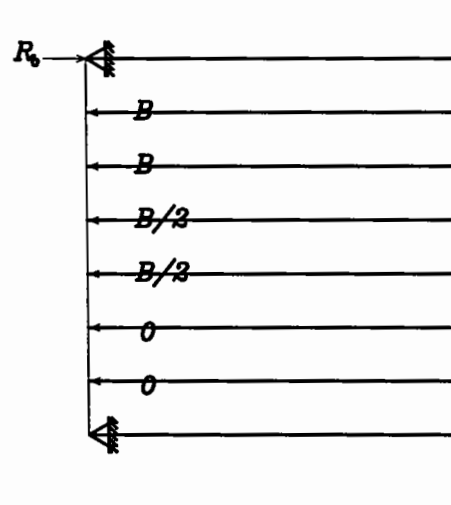
$$B = \frac{\text{Wheel Loads}}{7} = \dots \text{tons.}$$



Double track:



Triple track:



-check stresses on Concrete:

in check stresses on concrete, the effect of R_{D+L+I} , M_{wind} and $M_{Braking}$ are appear:

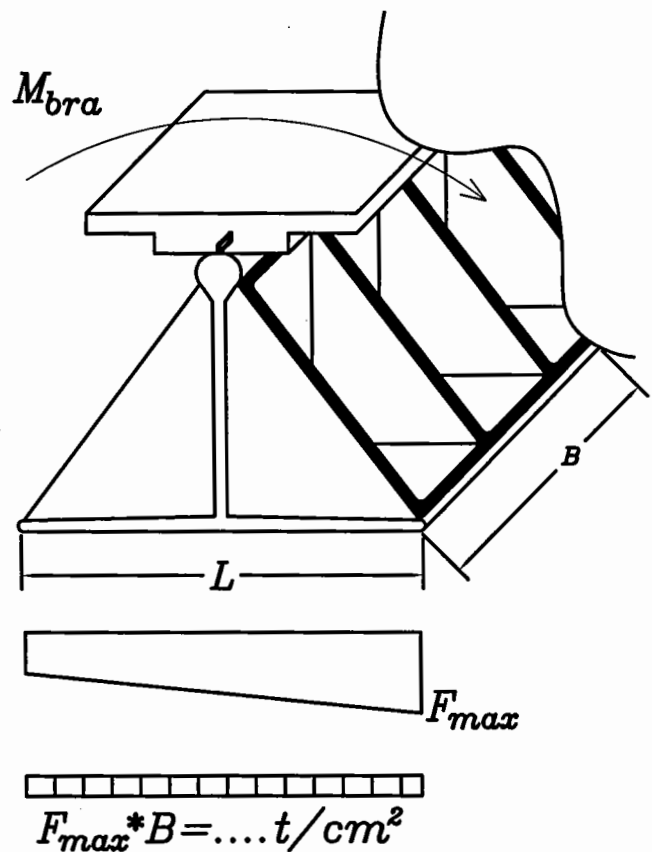
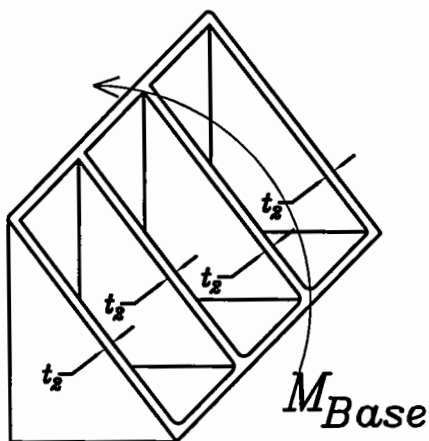
$$F_{max} = \frac{R}{L*B} + \frac{M_{wind}}{L*B^2/6} + \frac{M_{braking}}{L^2*B/6} = \dots\dots > 70 \text{Kg/cm}^2$$

$$F_{min} = \frac{R}{L*B} - \frac{M_{wind}}{L*B^2/6} - \frac{M_{braking}}{L^2*B/6} = \dots\dots > \text{Zero} \quad (\text{No Tension})$$

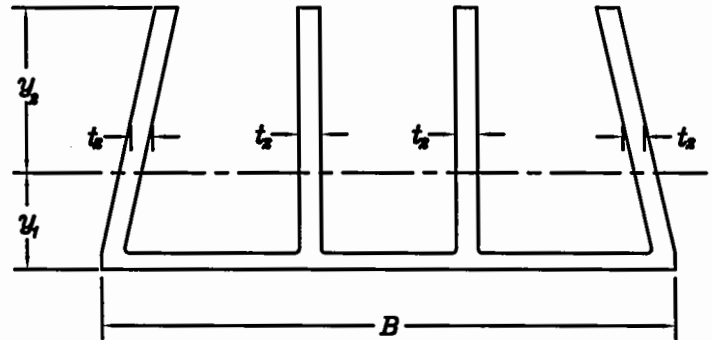
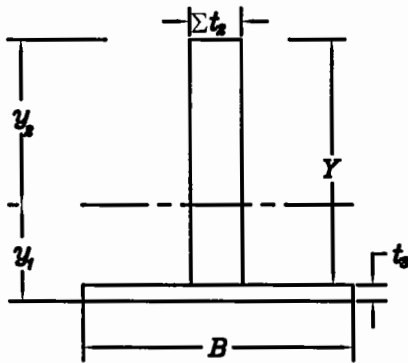
Where $L, B \longrightarrow$ Dimension of the Block.

-check stresses on Steel Block:

In check stresses on steel block, the effect of R_{D+L+I} and $M_{Braking}$ only while neglect the effect of wind load M_{wind}



The previous section can be translated into the following
Section



$$Y = H - t_s - t_{\text{sole plate}}$$

$H \rightarrow$ height of roller and hinge bearing.

$t_{\text{sole plate}} \rightarrow$ is the calculated thickness of sole plate.

$$\bar{y} = y_1 = \frac{B \cdot t_s \cdot t_s / 2 + y \cdot \Sigma t_s (y / 2 + t_s)}{B \cdot t_s + y \cdot \Sigma t_s} = \dots \text{ cm.}$$

Calculate I_x

$$I_x = \frac{B t_s^3}{12} + B t_s \left(y_1 - \frac{t_s}{2} \right)^2 + \frac{\Sigma t_s y^3}{12} + \Sigma t_s \cdot y \cdot (\bar{y} - y / 2 - t_s) = \dots \text{ cm}^4$$

$$F_{\text{max}} = \frac{R}{L \cdot B} + \frac{M_{\text{braking}}}{L^2 \cdot B / 6} = \dots \text{ t/cm}^2$$

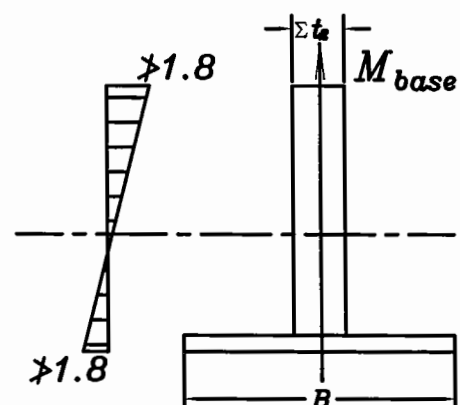
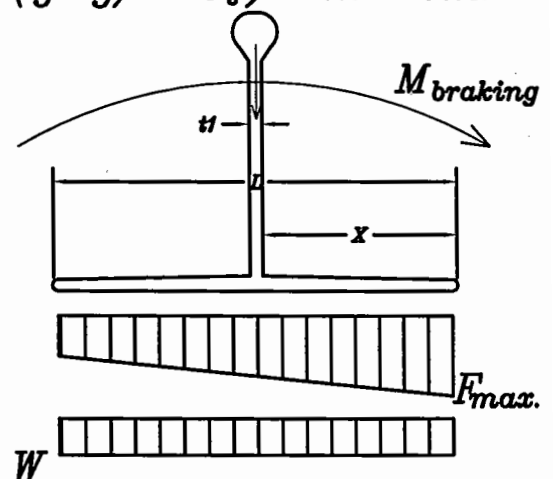
$$W = F_{\text{max}} \cdot B = \dots \text{ t/cm.}$$

$$X = \frac{L - t_1}{2}$$

$$M_{\text{base}} = \frac{W \cdot X^2}{2} = \dots \text{ cm.t.}$$

$$f = \frac{M_{\text{Plate}}}{I_x} (y_1 \text{ or } y_2) \geq 1.8 \text{ t/cm}^2$$

ايهما اكبر



Example(3):

Design Hinged support for a pony bridge of span 35m, equivalent L.L. (including impact) is 5 t/m' and equivalent D.L. is 3t/m' , the height of M.G. is 3.50m, the breadth of the bridge is 12.00m with 2m side walk each side, $h_{xg}=1.80m$, b , after curtailment is $\boxed{25cm}$, $F_{cu}=350Kg/cm^2$ with allowable stress = 75Kg/cm²

Solution:

$$W_t = 3 + 5 = 8t/m'$$

$$R_{D+LL+I} = \frac{8 \times 35}{2} = 140t.$$

$M_{wind} \rightarrow$ for case of loaded bridge

خلى بالك تم استعمال R_{D+LL+I} و سوف يتم استعمال قوى الفرمة وهذا
يعنى ان الكوبرى محمل.

$$X = 1.8 + 3 - 3.50 = 1.30m.$$

$$W_w = 0.1(3.5 + 1.3) = 0.48t/m'$$

$$R_w = 0.48 \times 35 / 2 = 8.40t$$

R_w is supported on 2 supports.

$$R = 8.4 / 2 = \boxed{4.2} \text{ on each support.}$$

Height of bearing is not given so it can be assumed = 40cm.

$$M_w = 4.2 \times 0.4 = \boxed{1.68m.t}$$

$$F_{br} = 1/4 \{ 60 + (35 - 6) \times 3 \times 0.50 \} = 25.9t < 90t.$$

$$X_1 = \text{side walk width} + 1/2(\text{width of main lane})$$

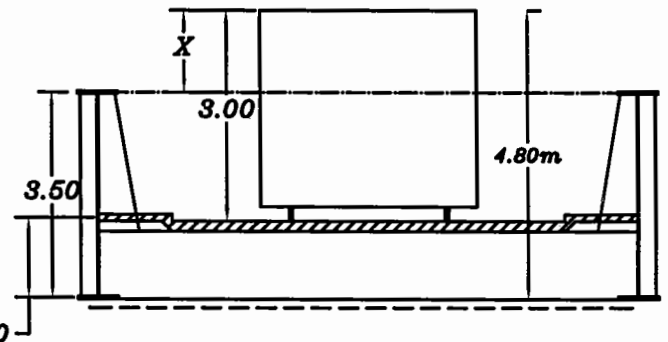
$$X_1 = 2 + 1.50 = 3.50m.$$

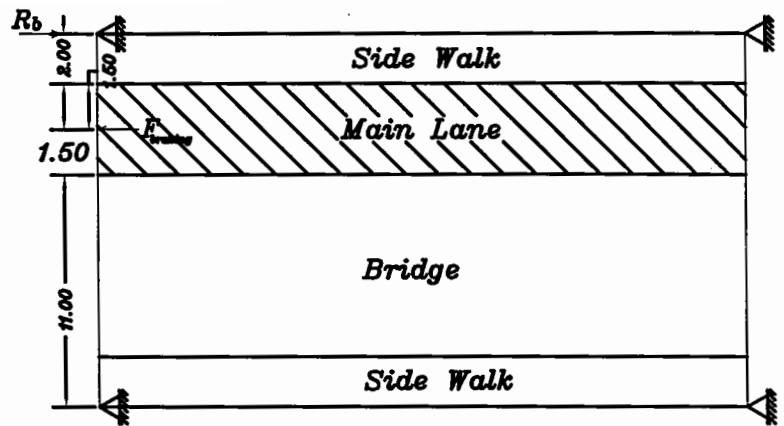
$$R_{br} = 25.9 \times \frac{12.5}{16} = \boxed{20.23t}$$

$$M_{br} = 20.23 \times 0.4 = \boxed{8.10m.t}$$

$$b_{roller} = 25 + 20 = 45cm.$$

$$b_{hinge} = 45 \times 1.2 = \boxed{55cm}$$





$$F_{\min}^{\max} = \frac{140}{55 \times 55} \pm \frac{168 \times 6}{55^3} \pm \frac{810 \times 6}{55^3} = 0.081 \text{ t/cm}^2$$

$$= 81 \text{ Kg/cm}^2 > 75 \text{ Kg/cm}^2 \text{ "Unsafe"}$$

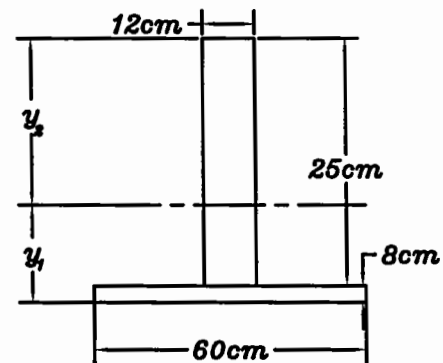
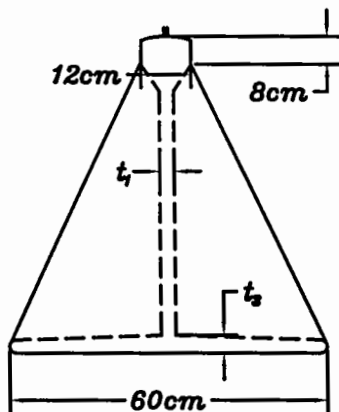
Assume base = 60*60.

$$F_{\min}^{\max} = \frac{140}{60 \times 60} \pm \frac{168 \times 6}{60^3} \pm \frac{810 \times 6}{60^3} = 0.066 \text{ t/cm}^2$$

$$F_{\max} = 66 \text{ Kg/cm} < 75 \text{ Kg/cm}$$

$$F_{\min} = 0.01 \text{ t/cm} > \text{Zero}$$

} O.K.



$$t_1 = \frac{40}{6-8} = 7.0 \text{ cm}$$

$$\Sigma t_2 = \frac{60}{5} = 12.00 \text{ cm}$$

$$t_3 = \frac{40}{5} = 8 \text{ cm}$$

Assume $t_{\text{sole plate}} = 7.0 \text{ cm}$.

$$\therefore Y = 40 - 7 - 8 = 25 \text{ cm}.$$

$$\bar{y} = \frac{60 \times 8 \times 4 + 25 \times 12 \times (8 + 25/2)}{60 \times 8 + 25 \times 12}$$

$$, y_1 = 10.34 \text{ cm}.$$

$$y_2 = 25 + 8 - 10.34 = 22.66 \text{ cm}.$$

$$I_x = 60 \times \frac{8^3}{12} + 60 \times 8 \times (10.34 - 4)^2 + \frac{12 \times 25^3}{12} + 12 \times 25 \times (25/2 + 8 - 10.34)^2$$

$$I_x = 68446 \text{ cm}^4$$

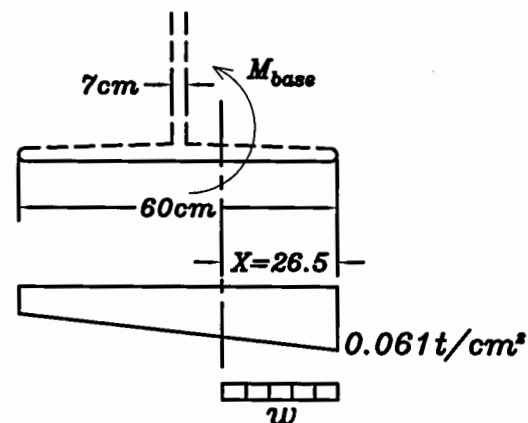
check stresses in cast steel plate:

$$X = (60 - 7) / 2 = 26.5 \text{ cm.}$$

$$F_{max} = \frac{140}{60 \cdot 60} + \frac{810 \cdot 6}{60^3} = 0.061 \text{ t/cm}^2$$

$$W = 0.061 \cdot 60 = 3.66 \text{ t/cm}$$

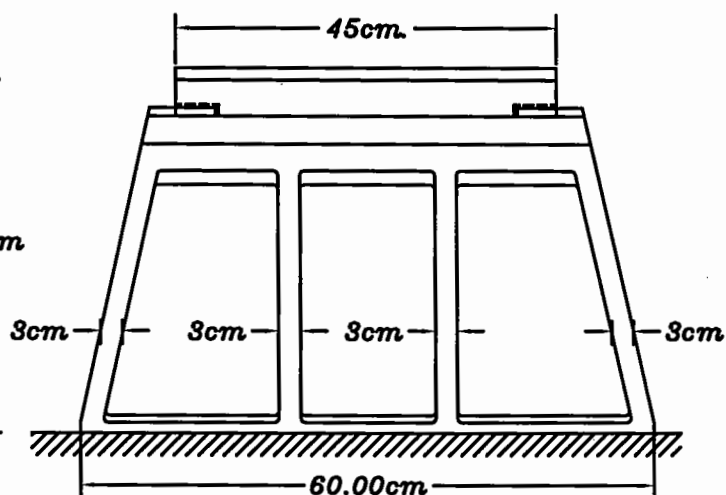
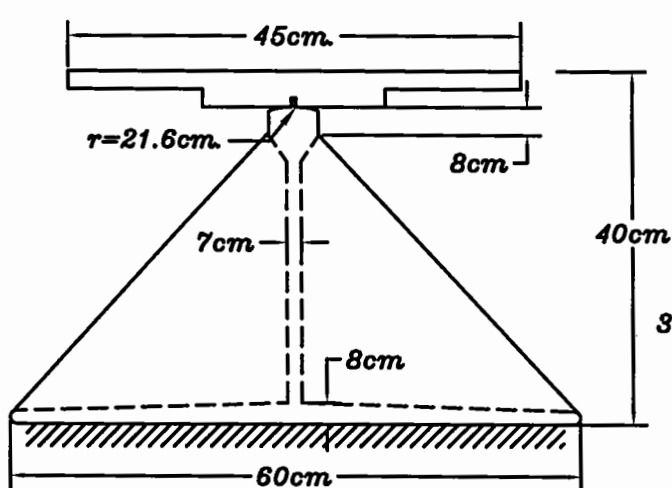
$$M = \frac{3.66 \cdot 26.5^2}{2} = 1285.17 \text{ cm.t.}$$



check

$$\frac{1285.117}{68446} \cdot 22.66 = 0.425 \text{ t/cm}^2 < 1.80 \text{ t/cm}^2$$

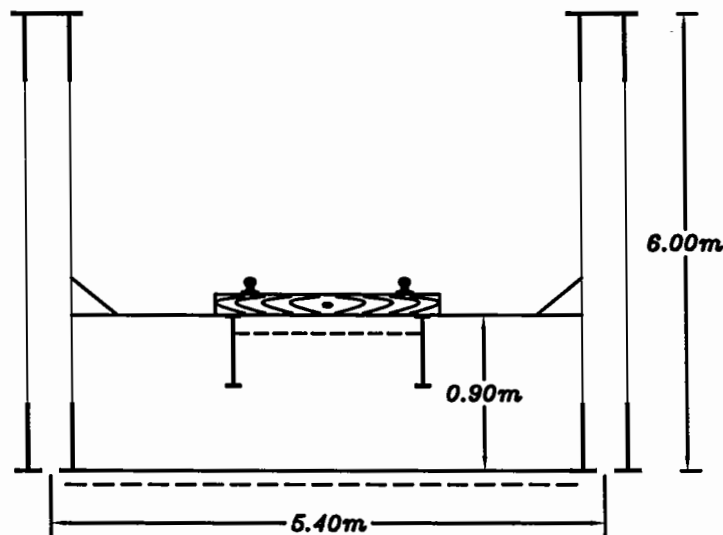
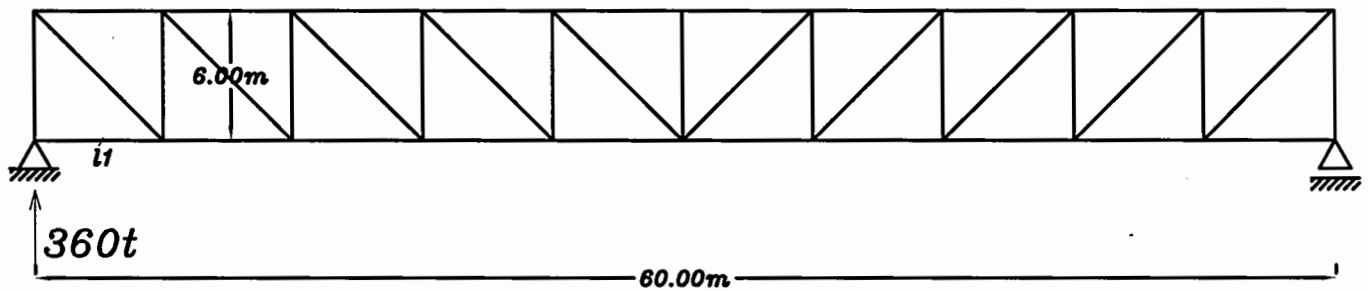
∴ O.K. Safe



Example(4):

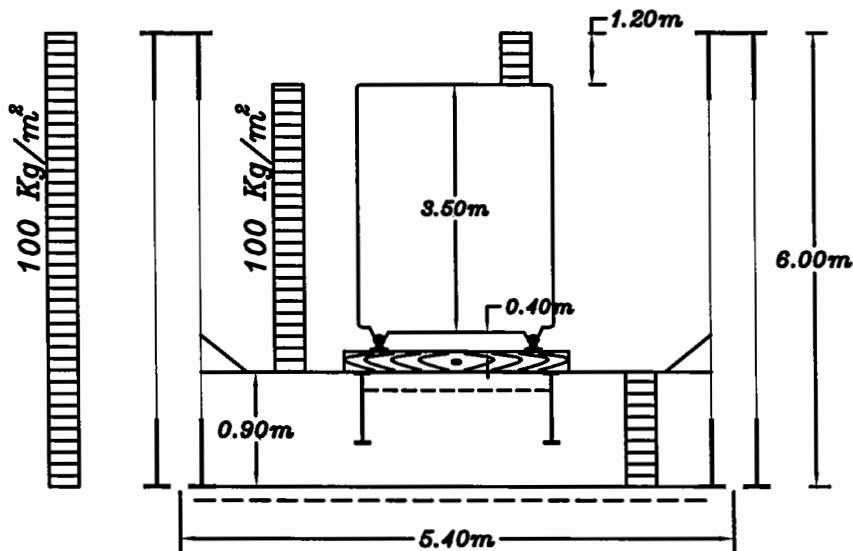
it is required to design hinged base for single track rail way pony bridge having a span of 60.00m and depth of main girder is 6.00m, the maximum reaction on the bearing due to total loads is 360t, roller bearing height is 52cm and dimension of roller base plate is 76cm*76cm

allowable concrete stress = 75 Kg/cm^2



take thickness of sole plate = 8.00cm

wind load



$$\text{case(3): } W_w = 0.1 * (0.5 * 6) + 0.1 * (3.5 + 0.4) + 0.1 * (0.5 * 2.1) = 0.79 \text{ t/m'}$$

$$R_s = \frac{0.79 * 60}{2} + 6t = 29.7t \quad (\text{loaded case})$$

$$R = 29.7 / 2 = \boxed{14.85t} \text{ on each support}$$

height of the bearing is = 0.52m (given)

$$M_w = 14.85t * 0.52 = \boxed{7.7m.t}$$

bracking load

$$B = \frac{\sum \text{Wheel Loads}}{7}$$

$$L = 9.00m(\text{Loc.}) + 8.40m(\text{Ten.}) + 10.50m(\text{Loc.}) + 8.40m(\text{Ten.}) = 36.3 \text{ m}$$

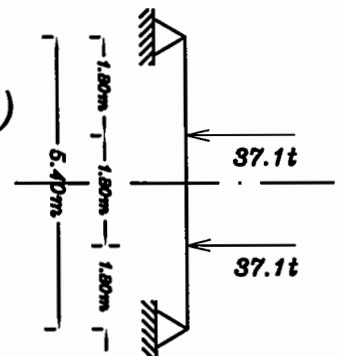
$$\text{no. of wagons} = \frac{60.00m - 36.30m}{12.00m} = 2$$

Wagon Length

$$\sum \text{Wheel Loads} = (50 + 40) * 2 + 40 * 2 = 260t$$

$$B = \frac{260}{7} = 37.1 \text{ t} \quad R_b = \boxed{37.1t}$$

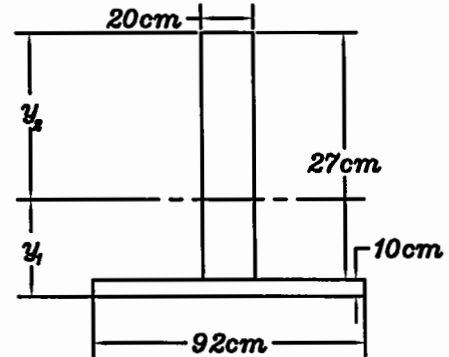
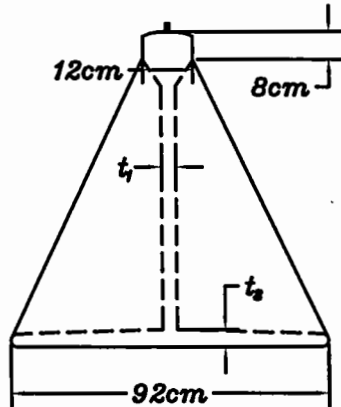
$$M_{\text{bracking}} = 37.1t * 0.52 = \boxed{19.3m.t}$$



hinged base dimensions = $1.2 \times 76\text{cm} = 92\text{cm}$

take hinged base to be $92 \times 92\text{cm}$

$$F_{\text{max}} = \frac{360}{92 \times 92} + \frac{770 \times 6}{92^3} + \frac{1930 \times 6}{92^3} = 0.063 \text{ t/cm}^2 = 0.021 \text{ t/cm}^2 \quad \left. \vphantom{\frac{360}{92 \times 92}} \right\} \text{O.K.}$$



$$t_1 = \frac{52}{6-8} = 7.0\text{cm}$$

$$\Sigma t_2 = \frac{92}{5} = 20.00\text{cm}$$

$$t_3 = \frac{52}{5} = 10\text{cm}$$

Assume $t_{\text{sole plate}} = 7.0\text{cm}$.

$$\therefore Y = 52 - 7 - 8 = 37\text{cm}.$$

$$\bar{y} = \frac{92 \times 10 \times 5 + 27 \times 20 \times (10 + 27/2)}{92 \times 10 + 27 \times 20}$$

$$, y_1 = 11.84\text{cm}.$$

$$y_2 = 27 + 10 - 11.84 = 25.16\text{cm}.$$

$$I_x = 92 \times \frac{10^3}{12} + 92 \times 10 \times (11.84 - 5)^2 + \frac{20 \times 27^3}{12} + 20 \times 27 \times (27/2 + 10 - 11.84)^2$$

$$I_x = 156930\text{cm}^4$$

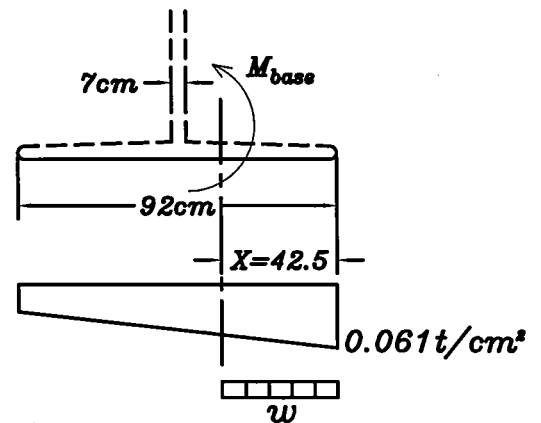
check stresses in cast steel plate:

$$X = (92 - 7) / 2 = 42.5 \text{ cm.}$$

$$F_{max} = \frac{360}{92 \cdot 92} + \frac{1930 \cdot 6}{92^3} = 0.057 \text{ t/cm}^2$$

$$W = 0.057 \cdot 92 = 5.28 \text{ t/cm}^2$$

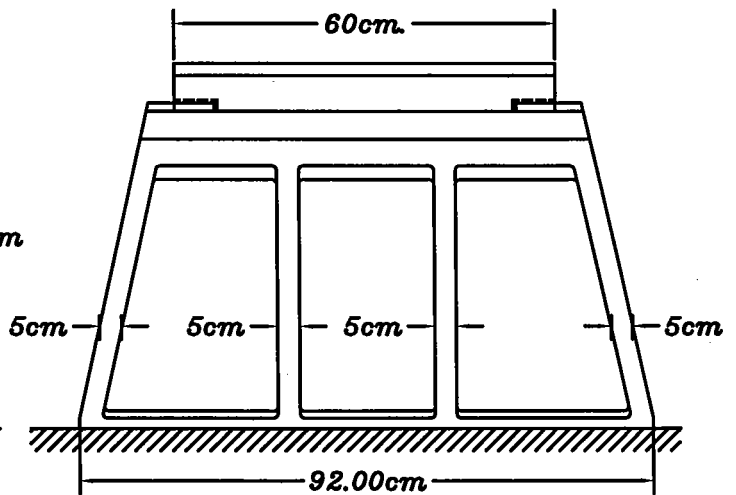
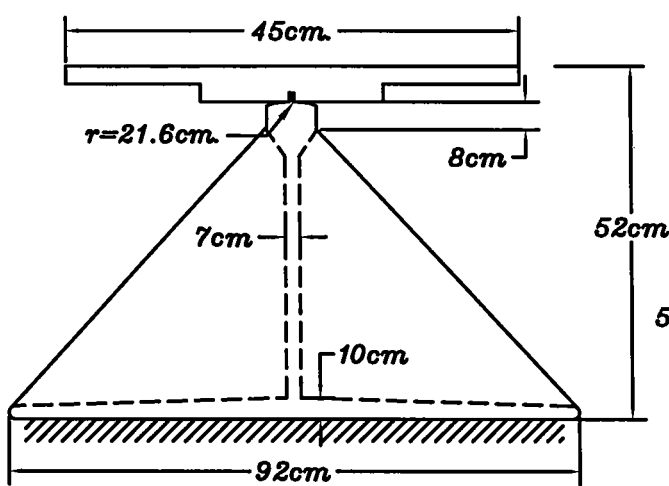
$$M = \frac{5.28 \cdot 42.5^2}{2} = 4768.5 \text{ cm.t.}$$

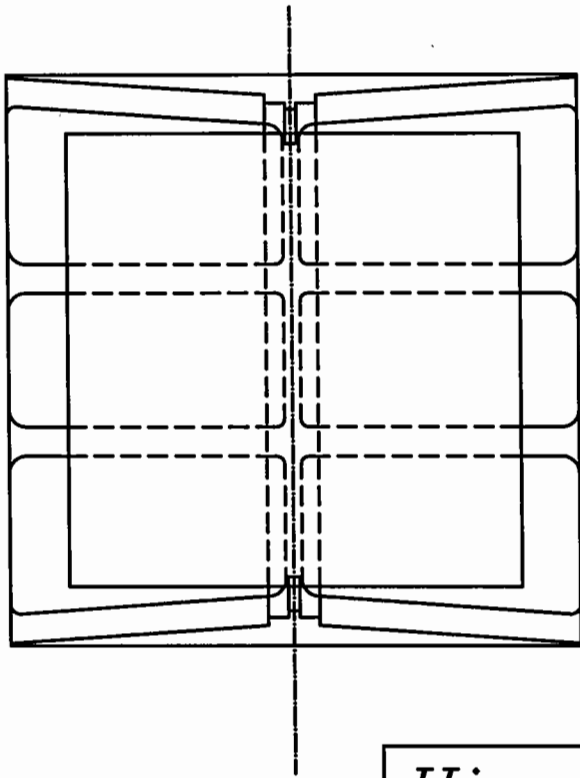
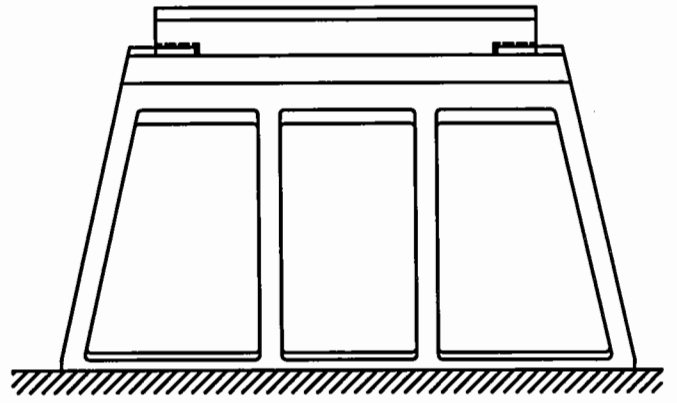
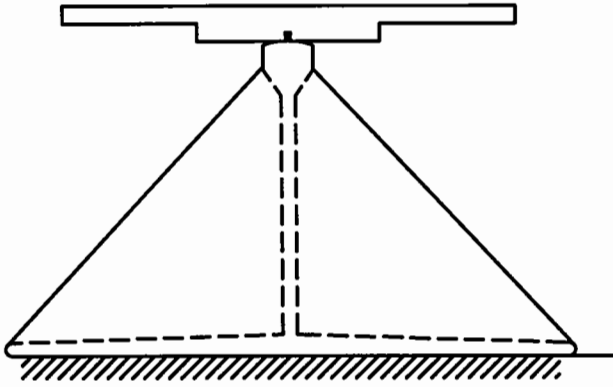


check

$$\frac{4768.5}{156930} \cdot 25.16 = 0.764 \text{ t/cm}^2 < 1.80 \text{ t/cm}^2$$

∴ O.K. Safe





Hinged Details